I-95 Corridor CoalitionVehicle Probe Project:Update on Validation of Arterial Probe Data

Summary Report



Prepared for:

I-95 Corridor Coalition

Sponsored by:

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Acknowledgements:

The research team would like to thank Coalition member states and their respective state highway officials for helping facilitate data collection for the validation efforts used in this report.

I-95 Corridor Coalition Vehicle Probe Project Evaluation – Summary of Arterial Reports Vendors: HERE, INRIX, TOMTOM

September 2019

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Executive Summary

At the beginning of 2013 the I-95 Corridor Coalition (I-95 CC) directed the Vehicle Probe Project (VPP) validation team to begin focusing on signalized arterial roadways, seeking to understand whether probe data – which was previously shown to be accurate on freeways – performed similarly on arterials. In response, the VPP validation team conducted nine data validation efforts along 14 corridors within the Mid-Atlantic region between April 2013 and June 2014. While additional analysis techniques were developed to evaluate arterial data quality, the same approach was used to obtain ground truth data; Wireless re-identification traffic monitoring (WRTM) technology (Bluetooth and/or Wi-Fi) equipment was deployed at strategic locations along the selected road segments and used to obtain direct measurement of travel times from a sample of vehicles, serving as a reference for comparison with VPP reported speeds.

After the initial report was written, the Vehicle Probe Project entered a second phase (i.e., VPPII), which introduced a probe data marketplace with three vendors: HERE, INRIX, and TomTom. Between 2014 and 2018, 13 data collection activities were carried out on 23 corridors within the mid-Atlantic region for all three vendors using a similar evaluation procedure. This report provides an update to the original arterial report, quantifying the accuracy of VPPII data across 13 additional arterial datasets and two additional vendors, and comparing these results to the original VPPI case studies. These results indicate that probe data performance on arterials has improved since the original analysis in 2014. In particular:

- Performance on the traditional analysis has improved, and VPPII probe data for all three vendors is much more consistently within contract specifications for traditional error measures (i.e., AASE and SEB) than was observed in the original VPPI case studies in 2013-2014. This is improvement is observable both when comparing vendor speeds to the ground truth Mean and Standard Error of the Mean (SEM) band speeds.
- Performance on the slowdown analysis the current preferred way to quantify operational performance on arterials improved dramatically relative to the VPPI levels for all three vendors and is less strongly linked to AADT and signal density. The average percentage of slowdowns missed by each vendor (an indicator of slowdown performance) across corridors dropped by over 50%, resulting in improved performance across a range of road characteristics.

Note that the case studies represent a finite range of road characteristics (e.g., AADT, signal density), and the conclusions may not generalize to conditions outside the observed

data (e.g., roads with extremely low volumes or high signal densities far beyond what was observed in the case studies). However, within the observed range of observed road conditions – particularly 0-3 traffic signals per mile and above 20k AADT, all three vendors typically perform at a level that is suitable for planning and many operational applications. This is a noticeable improvement from the results shared in the previous report, where performance was more closely linked to road characteristics and degraded significantly for signal densities over 2 signals per mile.

However, it is important to recognize that in contrast to freeways, **signalized arterials** have complex traffic patterns that cannot be fully captured the way vendors currently report data, which has not changed since the initial report was produced in 2015. One of the main challenges is that vendors report a single average speed for each time period, which cannot possibly capture bi-modal and other flow patterns that are common on arterials. Thus, even as probe data is becoming increasingly useful on higher signal density roads (as evidenced by strong slowdown analysis performance), it simultaneously cannot fully describe complex traffic conditions - especially during typical (i.e., non-slowdown) periods where there is more variation in speeds. Additionally, arterial probe data tends to consistently error towards faster speeds during congested periods – although the positive speed bias is less severe than was previously observed in the initial report.

Despite these fundamental issues, VPPII arterial speed data is more accurate than what was observed in the previous report, able to better capture traffic slowdown events, and suitable for use over a wider range of road characteristics. Based on these findings, the validation team recommends the following next steps:

- The I-95 Corridor Coalition should continue to evaluate probe data quality on arterials to benchmark vendor capabilities.
- Additional emphasis should be placed on refining analysis techniques for evaluating data quality on arterials.
- The I-95 Corridor Coalition should engage probe data vendors to discuss whether additional information can be reported on arterials

1. Background

The University of Maryland (UMD), under the direction of the I-95 Corridor Coalition (I-95 CC), has been responsible for evaluating the quality of Vehicle Probe Project (VPP) commercial probe data since the program began in 2008. The first phase of the project (VPPI) consisted of a single probe vendor – INRIX, and initially focused on quantifying data quality on freeway road segments. After spending the first few years developing methods and an intuition for data quality on freeways, the VPP validation team began shifting its attention to arterials.

This focus on arterials resulted in a dedicated data collection effort between April 2013 and June 2014, resulting in nine deployments across 14 arterial corridors in the Mid-Atlantic Region, and culminating in a comprehensive report summarizing the state of probe data quality on arterials. This initial arterial report used three analysis techniques: (1) the traditional validation analysis that had been used previous on freeways, which computes precision and bias error metrics to quantify performance, (2) a slowdown analysis to quantify the extent to which congestion events are captured , and (3) a distribution analysis to evaluate recurring congestion patterns. Based on the results, the VPP team summarized the overall quality of data, and provided insights on which methods were most appropriate, as well as directions for future work. These findings are summarized below for convenience.

After the initial report was written, the Vehicle Probe Project entered a second phase (i.e., VPPII), which introduced a probe data marketplace with three vendors: HERE, INRIX, and TomTom. Between 2014 and 2018, 13 arterial data collections were carried out on 23 corridors within the mid-Atlantic region for all three vendors, and the data was evaluated in a similar manner as the previous report. This report summarizes the performance for all three vendors in comparison to previous result, focusing on the traditional and slowdown analysis methods.

Summary of previous arterial validation report findings

At high level, the previous arterial report – through various analysis techniques - found that probe data performance correlated most strongly with traffic signal density; higher traffic signal density tended to correspond with lower accuracy. To a lesser extent, it found that higher traffic volumes were positively correlated with probe data accuracy but emphasized that even high volumes could not overcome the challenges posed by close signal spacing. Based on the observed probe performance across a range of signal

densities and Average Annual Daily Traffic (AADT) values, it made the following toplevel recommendations:

- Probe data was recommended for operations and performance measures for signal density values less than or equal to 1 signal per mile and AADT values of at least 40,000.
- It was recommended that probe data should be used with caution for signal densities between 1-2 signals per mile and AADT values between 20k-40k.
- It was NOT recommended for signal density values above 2 signals per mile and AADT values below 20,000.

Additionally, the report also noted some fundamental issues that were observed through the analysis – particularly that probe data tends to over-report speeds during congested periods, and that complex flow patterns observed on arterials cannot be fully characterized with the average speed values reported by vendors. For example, bimodal speed distributions are sometimes observed in the ground truth data but cannot be captured by a single VPP speed value that lacks information about speed variation. In the case of multiple valid observed speeds, the VPP speed tended to track the higher speed – a phenomenon referred to as optimistic bias.

It also commented on the suitability of the analysis techniques used to evaluate VPP data quality on arterials. It noted that the traditional analysis that has been used extensively for prior VPP validation cannot fully characterize performance on arterials; it is possible to achieve acceptable error metric values without performing well - particularly when VPP data is assessed against SEM band. It also indicated that the slowdown analysis provided the best insight into whether VPP data accurately captured traffic conditions, and that the distribution analysis was well suited for quantifying whether probe data can capture recurrent congestion patterns.

Based on the findings, it made recommendations to the I-95 Corridor Coalition about how to proceed, which included continuing to focus on data quality on arterials, utilizing and building upon the analysis techniques in the report, and engaging data providers and researchers to figure out how probe data can be reported in a more meaningful way.

2. Case Study Locations

Figure 1 shows the locations of new (i.e., VPPII) case study locations used to evaluate arterial probe data in this report, while additional information about all locations can be found Tables 1 and 2. Table 1 summarizes the VPPI case studies from April 2013 through June 2014 (i.e., the subject of the previous report), while Table 2 summarizes VPPII case studies from December 2014 through November 2018. Both tables contain the road name, code that was assigned to each corridor, validation date, the bi-directional traffic volume as reported by the Highway Performance management System (HPMS), and signal density.

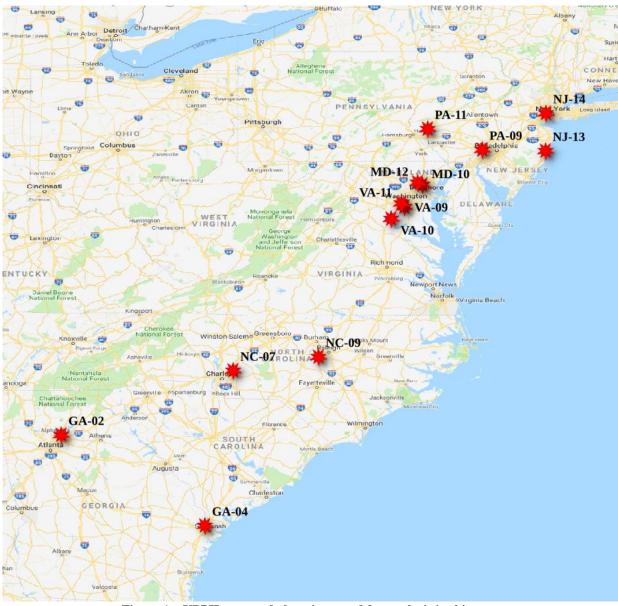


Figure 1 – VPPII case study locations used for analysis in this report.

Table 1 - VPPI case study locations and attributes

Case Study Number	Data Set (State-ID#)	Road #	Corridor Code	Validation Date Span	Average AADT (in 1000)	Average Signal Density
1	NC-06	NC-55	1	Apr 30-May 13, 2013	25.0	2.1
2	MD 07	MD-355	2a	July 6-20,	44.0	3.9
	MD-07	MD-586	2b	2013	34.0	3.1
		US-1	3a	6 10 04	70.0	0.7
3	NJ-11	NJ-42	3b	Sep 10 - 24, 2013	48.0	1.8
		US-130	3c	2013	42.0	2.0
4	NJ-12	NJ-38	4a	Nov 5-19,	46.0	1.8
4		NJ-73	4b	2013	52.0	1.7
5	PA-05	US-1	5a	Dec 3 - 14,	45.0	4.1
3	1 A-03	US-322	5c	2013	25.0	0.5
6	PA-06	PA-611	6a	Jan 9 - 22,	27.0	3.3
0	FA-06	PA-611	6b	2014	21.0	11.5
		VA-7	7a	A 11 F 16	56.0	1.9
7	VA-07	VA-7	7b	April 5-16, 2014	55.0	1.6
		US-29	7c	2014	21.0	5.0
8	VA-08	US-29	8	May 8-19, 2014	33.0	3.6
9	MD 00	MD-140	9a	Jun 5-17,	31.0	3.9
7	MD-08	1/11/0-1440	9b	2014	42.0	1.2

Table 2 - VPPII case study locations and attributes

Table 2 - VPPH case study locations and attributes								
Case Study Number	Data Set (State-ID#)	Road #	Corridor Code	Validation Date Span	Average AADT (in 1000)	Average Signal Density		
11	VA-09	US-1	11	Dec 4 - 18, 2014	36.0	2.9		
12	VA-10	US-1	12	Jan 15 - 28, 2015	22.0	1.2		
13	NJ-13	NJ-37	13	June 30-July 12, 2015	39.8	1.0		
4.4	NGOT	US-29	14a	Nov 11 - 25,	28.7	1.4		
14	NC-07	US-74	14b	2015	57.8	1.0		
		GA-141	15a	F.1.0.40	43.2	2.3		
15	GA-02	US-41	15b	Feb 3 - 18,	30.8	1.9		
		US-19	15c	2016	146.5	0.0		
16	MD 10	US-1	16a	Mar 25-Apr	29.2	2.2		
16	MD-10	US-29	16b	10, 2016	62.0	1.5		
		PA-3	17a	. 20	28.3	4.6		
17	PA-09	PA-23	17b	Apr 20- May 5, 2016	11.1	0.9		
		US-30	17c	May 3, 2016	23.4	4.6		
18	VA-11	US-50	18	Sep 26-Oct 7, 2016	52.9	2.8		
		US-1/9	19a	N. 46 20	74.1	1.9		
19	NJ-14	US-1	19b	Nov 16 - 28, 2017	90.4	0.9		
		US-9	19c	2017	75.5	0.4		
20	MD-12	US-40	20	Mar 19 - 30, 2018	40.3	2.4		
21	NC-09	NC-55	21	May 8 - 20, 2018	28.9	2.1		
		GA-21	22a		31.9	2.7		
22	GA-04	US-80	22b	Oct 23-Nov	22.2	2.3		
22	JA-04	E/W Bay St	22c	4, 2018	20.9	5.7		
23	PA-11	US-22	23	Nov 14 - 26, 2018	15.7	2.4		

3. Case Study Analysis Methods

Two analysis methods are used to evaluate each of the arterial case studies: (1) the traditional validation method, and (2) a slowdown analysis method. The traditional method evaluates the vendor data over all time (even if traffic conditions are uninteresting), whereas the slowdown analysis focuses specifically on traffic perturbations – an approach that makes it well-suited for evaluating operational performance. Note that the original arterial report contained a third approach (i.e., the sampled distribution method), but it is omitted here because it is no longer used regularly for evaluation purposes.

3.1 Traditional Validation Method

The traditional (also referred to as standard) validation analysis consists of comparing sampled ground truth (i.e., WRTM) speeds against vendor speeds over five-minute intervals and quantifying the discrepancy in terms of two error metrics defined in the contract specifications. This evaluation approach was originally created to evaluate VPP data on freeways, and over time has also been used on arterials by adjusting the speed bins to reflect typical speeds on arterial facilities.

WRTM speeds are summarized in terms of (space) mean speed and confidence band for each five-minute period. The WRTM mean speed is an estimate of mean speed for the entire traffic stream, while the confidence band accounts for uncertainty in the estimate based on the number of samples and variability of observed speeds. Several statistical measures were initially evaluated to define the width of this uncertainty band, all of which are described and reported in the original report. Ultimately, the standard error of the mean (SEM) measure was selected due to its simplicity and sensitivity to both variability and number of observations used for calculations. The SEM is calculated as the standard deviation (SD) of the ground truth measurements divided by the square root of the number of ground truth data points (n) taken for a given time. In other words, SEM = $\frac{SD_{WRTM}}{\sqrt{n}}$. The confidence band based on this statistic (i.e., the SEM band) narrows when there is a higher degree of confidence in the WRTM estimate (i.e., based on more observations or less variation) and widens when there is less confidence, seeking to capture the true mean about 95% of the time.

A statistical analysis of the data is conducted for four defined speed bins, where each five-minute interval is associated with a speed bin based on its corresponding ground truth space-mean speed (0-15 mph, 15-30 mph, 30-45 mph, 45+ mph for arterials; 0-30 mph, 30-45 mph, 45-60 mph, 60+ mph for freeways). Reported probe speeds are compared to both the space-mean and SEM band ground truth speed for each five-minute time interval, and

the discrepancies are quantified in terms of two error metrics: Average Absolute Speed Error (AASE) and Speed Error Bias (SEB), which are reported separately for each speed bin. According to contract specifications, AASE and SEB values must be within 10 mph and 5 mph, respectively, when compared with the SEM band.

Average Absolute Speed Error (AASE)

AASE is calculated by summing up the absolute difference between probe vendor speeds (S_P) and ground truth speeds (S_{GT}) for each time interval and taking the average over n observations. That is, AASE $=\frac{1}{n}\sum_{i=1}^{n}|S_P-S_{GT}|$. Because the absolute value is used, positive and negative errors cannot cancel, and the result is always positive.

Speed Error Bias (SEB)

Speed Error Bias is calculated similarly to AASE, with the difference that the absolute value of the errors is not taken. In other words, SEB = $\frac{1}{n}\sum_{i=1}^{n}S_{P} - S_{GT}$. Thus, positive and negative errors can cancel each other out, and the resulting value can provide insight into whether there is a consistent positive or negative error.

A sample of corridor-level results from a recent validation is shown in Table 3.

Table 3 - Example of Traditional Error Metrics

Traditional \	Traditional Validation Metrics Example										
		solute Speed 10mph)	-	rror Bias nph)	Number of 5						
Speed Bin	Comparison with SEM Band	Comparison with Mean	Comparison with SEM Band	Comparison with Mean	Minute Samples						
0-15 MPH	1.64	3.9	1.44	3.06	1587						
15-25 MPH	1.49	5.15	1.03	3.48	5316						
25-35 MPH	1.15	5.33	0.33	2.66	11125						
>35 MPH	1.58	4.88	-1.38	-2.74	19731						
All Speeds	1.44	5.01	-0.42	-0.03	37759						

It should be noted that the traditional methodology was originally designed for freeway analysis and has been adjusted over time to accommodate arterial roads. However, as the original report pointed out, the SEB band tends to be much larger on signalized arterials – sometimes resulting in vendor speeds that are within the large confidence interval, but do not accurately reflect traffic characteristics. Part of the challenge is systemic; vendors report a single average speed for each time period, but complex traffic flow often has a wide range of valid speeds, and in some cases two distinct modes, which cannot possibly be captured by a single value.

It has been suggested that comparing vendor data with the mean ground truth value may be a better solution, but this approach is also imperfect in the case of widely-varying or bimodal speeds. Accordingly, the traditional methodology continues to compare vendor data to both the mean and SEM band ground truth speeds, and proper interpretation of the traditional validation metrics should take these factors into consideration.

3.2 Slowdown Analysis Method

The slowdown analysis is an offshoot of the traditional analysis, developed to provide a more intuitive measure of probe data's ability to capture congestion events. The definition of a slowdown in this context is when traffic speeds (as identified by ground truth WRTM sensors) decrease by at least 15 mph for a period of one hour or more. On slower speed arterials, the threshold may be reduced to a reduction in speed of 10 mph, and the duration of 30 minutes or greater.

An analyst visually compares ground truth and vendor speeds for each slowdown event, focusing on how well the vendor data captures the magnitude and duration of the speed reduction. Each slowdown is ultimately classified as 'Fully Captured', 'Partially Captured', or 'Failed to Capture' according to the following rules:

• A **Fully Captured slowdown** indicates that the probe data accurately characterized both the reduction in speed, and duration of the slowdown. The error in speed reduction or duration cannot exceed 20%.

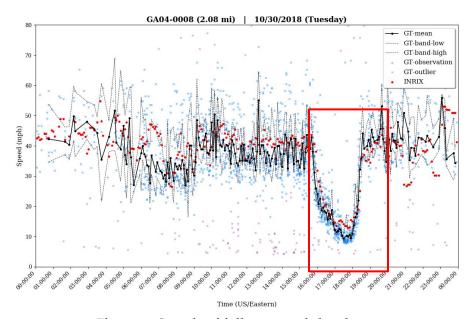


Figure 2 - Sample of fully captured slowdown.

• A **Partially Captured slowdown** indicates that the probe data reported a significant disruption to traffic, but the extent of speed reduction or duration of time were in error by more than 20%.

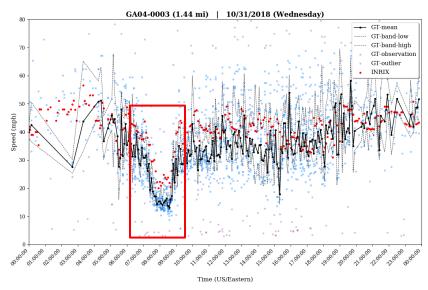


Figure 3- Sample of partially captured slowdown.

• **Failed to Capture** indicates that the probe data either completely missed the slowdown, or the extent of speed reduction or duration of the event were significant in error such that the slowdown would not be interpreted as a significant disruption to traffic.

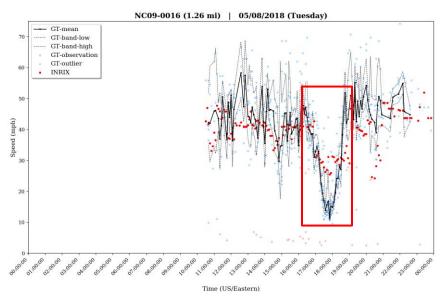


Figure 4 - Sample of failed to capture slowdown.

The slowdown analysis is a manual process that requires human judgment to classify each slowdown event. However, the previous arterial report's analysis indicated that this method was the most capable of quantifying vendor performance – particularly for operational purposes.

4. Summary of Case Study Results

Both analysis methods (i.e., traditional validation and slowdown analysis) were applied to the data collected from all three vendors along the arterial corridors for the 13 new case studies. The results are summarized and discussed in this section, and tables containing detailed information can be found in the Appendix.

4.1 Traditional Validation

As described in Section 3.1, the traditional analysis produces AASE and SEB error metrics that compare vendor speeds to both the ground truth mean speed and standard error of the mean (SEM) band (i.e., a proxy for the 95% confidence interval). The error metrics from the original arterial report (i.e., old VPPI case studies) are reported in Table A.1, while the results from the new case studies can be found in Tables A.2 (VPPII Vendor 1), A.3 (VPPII Vendor 2), and A.4 (VPPII Vendor 3). Each table contains both AASE and SEB scores separated by a slash ('/'), where the number preceding the slash is the metric assessed against the mean of the ground truth data and the number after the slash is the same metric assessed against the SEM band.

Compliance with Contract Specifications

Comparisons assessed against the SEM band have historically been used for evaluating vendor performance, although it is worth noting that the contract specifications were initially designed with freeways in mind. Instances where vendors did not meet these specifications (i.e., where vendor speeds were not within 10 MPH for AASE or ±5 MPH for SEB when compared to the SEM band in each speed bin) are colored red in tables A.1-A.4. These tables show clear improvement in vendor compliance between VPPI and VPPII case studies; data was within specification for 93% of AASE and 67% of SEB scores during VPPI, and these percentages improved to 100% of AASE and 83%-98% (depending on vendor) of SEB scores during VPPII.

Figures 5-6 provide additional insight into vendor compliance, with Figure 5 focusing on AASE, and Figure 6 on SEB. Both plots show the compliance percentages for each vendor for each speed bin when vendor speeds are compared to the mean (left) and SEM band (right). Even though the mean has not been used historically for making "pass/fail" determinations, it is instructive to understand how the vendor speeds compare to the (space) mean ground truth speed - particularly because arterials tend to have large variations in traffic speed, which can at times obscure how well vendor data is performing.

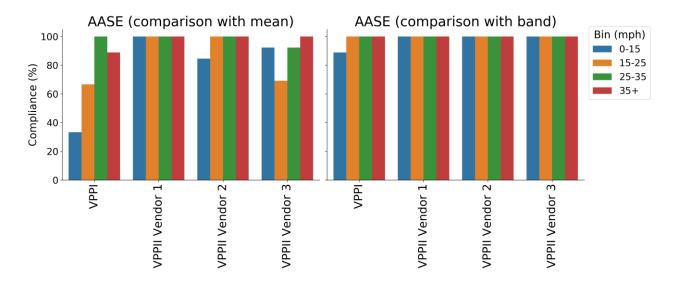


Figure 5 – AASE compliance percentages for VPPI and VPPII case studies.

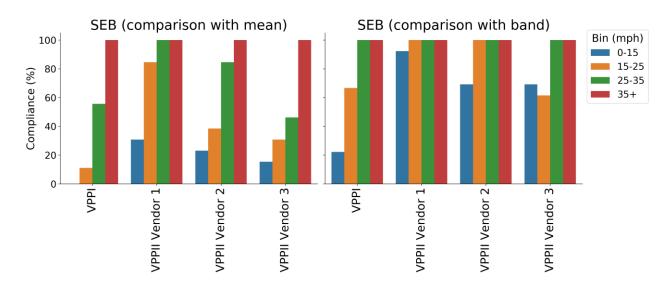


Figure 6 – SEB compliance percentages for VPPI and VPPII case studies.

First consider Figure 5, and note the results for VPPI (i.e., the old case studies). Despite strong performance when compared to the SEM band, the compliance rate when compared to the mean is much lower for the lowest two speed bins. However, the results for all three vendors in the VPPII case studies show much better performance when compared to the mean – particularly for the two lowest speed bins that are known to be more challenging. Figure 6 shows similar results, except the discrepancy in compliance when comparing vendor data to the mean and SEM band is even more pronounced. Nonetheless, despite rather low compliance rates when compared to the mean, all three vendors do show noticeable improvement relative to the VPPI case studies.

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Overall Trends

Finally, it is useful to visually compare the error metrics contained in A.1-A.4 to identify trends between VPPI and VPPII case studies - shown in Figures 7 (AASE) and 8 (SEB). In each bar chart, the bar represents the mean error metric value for all validations in that category, while the black line shows the value of the standard deviation. For example, in Figure 7, the average AASE when compared to the mean (left plot) in old case studies (i.e., VPPI) for the 0-15 mph (blue bar) is just under 12 mph with a standard deviation around 5 mph.

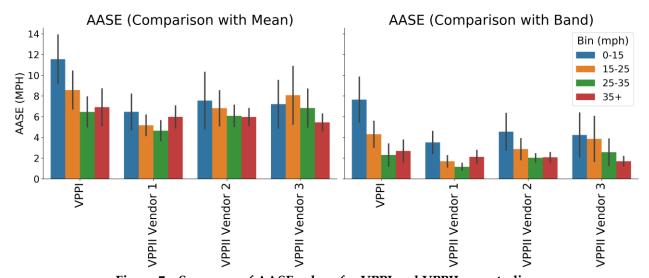


Figure 7 – Summary of AASE values for VPPI and VPPII case studies.

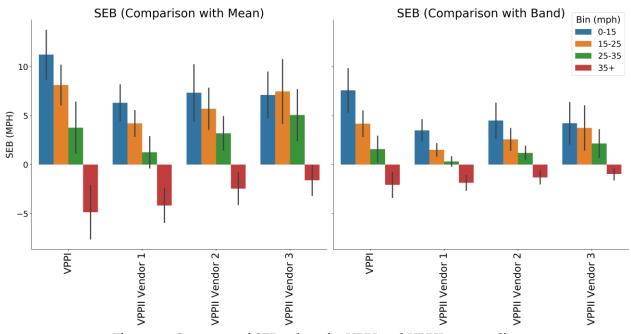


Figure 8 - Summary of SEB values for VPPI and VPPII case studies.

A few general trends are evident from Figures 7 and 8. First, all three vendors are, on average, producing AASE and SEB error metrics that are better than what was observed initially in VPPI – both when compared to the mean and SEM band. Given that comparisons with the SEM band are not as meaningful when there are large speed variations on the road, improvements when compared to the mean are encouraging, and indicate that the data quality has improved. Additionally, while the lowest two speed bins still appear to be the most challenging for vendors, the extent to which this is the case is less for the new case studies. Finally, on average, all three vendors tend to overestimate speeds in the 0-15, 15-25 and 25-35 speed bins (i.e., positive bias), and underestimate them in 35+ speed bin (i.e., negative bias). However, the magnitude of these biases is much lower in the VPPII case studies for all three vendors than for the old case study. This last point relates to findings from the previous arterial report, which observed that probe data tends to overestimate traffic speed during congestion. While this appears to still be the case for all three vendors, the phenomenon is less pronounced.

4.2 Slowdown Analysis

The results of the slowdown analysis for old and new case studies are summarized in Tables A.5 and A.6, respectively. Each table shows the total number of slowdowns and the percent fully captured, partially captured and failed to capture for each corridor within the case studies, as well as roadway information that was previously found to correlate with slowdown performance. The following sections analyze the data from various perspectives.

Overall Performance

First, it is useful to compute the overall slowdown performance across all observed slowdowns (regardless of corridor), a perspective that gives a high-level view of overall slowdown accuracy. A key advantage to this approach is that it gives equal weight to all slowdown events, but a potential disadvantage is that the overall accuracy may be overly influenced by observations on a few corridors, which may not be representative of all road types. To this point, note that the number of slowdowns observed on old VPPI case studies ranges from 0-101 with mean = 35.2 and median = 20, and from 0-140 with mean = 29.3 and median = 17 on new VPPII case studies. Thus, for both VPPI and VPPII case studies, there is significant variation in the number of slowdowns, meaning that some corridors contribute much more than others to the overall performance.

Table 4 summarizes the overall slowdown performance for each vendor across all case studies using the data from Tables A.5 and A.6. It clearly shows that all vendors performed much better on the slowdown analysis in new VPPII case studies than the VPPI

data showed on old case studies; the percentage of fully captured slowdowns improved from 33% to 59-66% (depending on the vendor), and percentage of failed to capture dropped from 25% to 6-10%. Furthermore, it highlights the fact that all three vendors are generally within 5% of one another, indicating that the slowdown performance differences are minor at the aggregate level. However, because these results consider all slowdowns – which are not evenly distributed among case studies, these results can be heavily influenced by a few corridors that may not be representative of all road geometries.

Table 4 - Overall slowdown analysis results across all corridors for VPPI and VPPII case studies

	Total Slowdowns	Fully Captured (%)	Partially Captured (%)	Failed to Capture (%)	Missing Data (%)
VPPI	670	32.7	41.9	25.4	0.0
VPPII - Vendor 1	674	65.6	24.2	9.6	0.6
VPPII – Vendor 2	674	59.2	30.6	9.9	0.3
VPPII – Vendor 3	674	61.1	29.2	5.9	3.7

Performance by Corridor

A second perspective is to compute slowdown performance independently on each case study / corridor, as was done in the previous report. An advantage of this approach is that it considers the varying road characteristics (e.g., signal density, AADT) associated with each corridor, which may give better insight into "typical" performance on a range of road geometries, and can be used to understand the conditions under which the data is adequate. Furthermore, the results are not dominated by a single corridor with many more slowdowns than the others, as can be the case when looking at overall performance. However, the primary disadvantage of computing the accuracy separately for each case study is that the slowdown performance on each corridor is equally weighted regardless of the number of slowdowns observed. Since the number of slowdowns varies dramatically across corridors, corridors with very few observations – where a single slowdown classification can significantly change the accuracy – may produce results that are less reliable than those with more slowdowns.

When considering the results from a corridor level perspective, a minimum slowdown threshold was added to omit corridors with less than a minimum number of slowdowns. This parameter was chosen to be 5 – around the 15-20th percentile in the distribution of slowdowns on new case studies – and was introduced to try to prevent case studies with extremely few opportunities for slowdown classification from being over-weighted in the results. Please note that this filtering approach was not conducted in the previous report but has been applied to the old VPPI case studies in this report for uniformity with the new VPPII results.

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Figure 9 summarizes the distribution of slowdown analysis performance across old and new case studies via boxplots. The left set of boxplots in Figure 9 focuses on the percentage of slowdowns that are "Fully Captured", while the right set summarizes the percentage that "Failed to Capture" slowdowns – both of which show that all three vendors' ability to fully capture slowdowns (and similarly avoid missing slowdowns) has improved significantly in since the initial arterial report was produced.

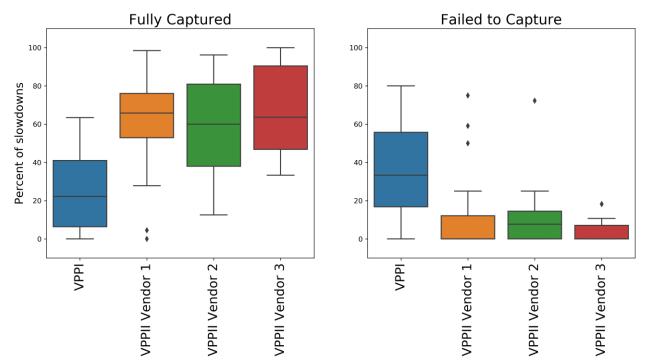


Figure 9 – Summary of slowdown analysis performance for VPPI and VPPII studies.

The key summary statistics for "Failed to Capture" – used extensively in the original arterial report as a strong indicator of arterial performance – are summarized in Table 5 and correspond to the right set of box plots in Figure 9. Notably, the average, median, standard deviation and percentiles of new case studies for all three vendors improved appreciably over the old case studies. For example, old case studies failed to capture slowdowns 37% of the time on average, which is now less than 14% for all three vendors (13.3%, 11.4%, and 4.2%). Furthermore, the improvement in slowdown performance (as measured by percentage of failed to capture slowdowns) was also corroborated statistically via hypothesis testing; the difference in mean "Failed to capture" percentages between new and old case studies was statistically significant for all three vendors.

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Table 5 - Statistics of failed to capture slowdowns across VPPI and VPPII case studies

Summary Statistics	% Failed to Capture Slowdowns						
Across Case Studies	VPPI	VPPII (Vendor 1)	VPPII (Vendor 2)	VPPII (Vendor 3)			
Mean	37.1	13.3	11.4	4.2			
Std. Deviation	24.9	22.8	16.9	6.1			
Percentile-25	16.8	0.0	0.0	0.0			
Percentile-50	33.3	0.0	7.7	0.0			
Percentile-75	55.8	12.1	14.5	7.1			

Additionally, it is instructive to see how slowdown analysis performance across corridors corresponds to road characteristics. The previous arterial report found a positive correlation between signal density and the percentage of "Failed to capture" slowdowns, and to a lesser extent a negative correlation between AADT and the percentage "Failed to capture" slowdowns – leading to recommendations about where probe data may be considered trustworthy. A similar analysis that includes new case studies is repeated here; Figures 10 and 11 show regression plots comparing "Failed to Capture" slowdowns to signal density and AADT, respectively, while Tables 6 and 7 summarize the results obtained via simple linear regression. It should be noted that most data points are within a limited range of road characteristics in the new case studies, and care must be taken when making judgements beyond that region (i.e., the trend lines beyond these regions do not actually reflect observed results). Along these lines, the data points from old and new case studies are based on slightly different road characteristic ranges, so the corresponding regression coefficients may not be directly comparable. Nonetheless, this approach allows general trends to be explored.

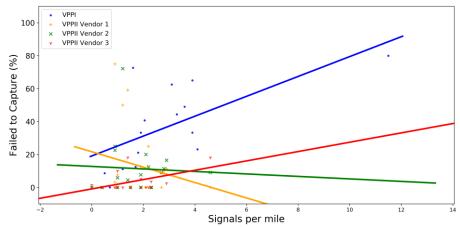


Figure 10 - Percent of failed to capture slowdowns versus signal density.

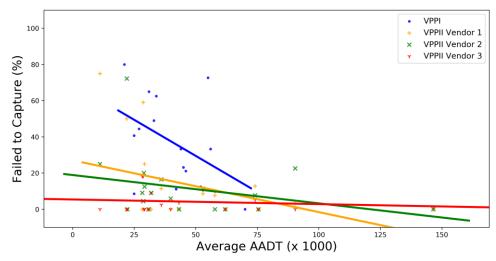


Figure 11 - Percent of failed to capture slowdowns versus average AADT.

Table 6 – Simple linear regression results for slowdown performance based on signal density.

0/ Eatladta Cantuma	Signal Density		(Interce	ept)	R^2	# Corridors	
% Failed to Capture	Coefficient	p-val	Coefficient	p-val	ĸ	# Corridors	
VPPI	0.060	0.01	0.19	0.03	0.41	15	
VPPII Vendor 1	-0.047	0.37	0.22	0.05	0.05	19	
VPPII Vendor 2	-0.008	0.85	0.13	0.13	0.00	19	
VPPII Vendor 3	0.028	0.03	-0.01	0.72	0.24	19	

Table 7 – Simple linear regression results for slowdown performance based on AADT.

0/ Failad to Contains	AADT		(Interce	ept)	R^2	# Corridors	
% Failed to Capture	Coefficient	p-val	Coefficient	p-val	K-	# Corridors	
VPPI	-0.008	0.09	0.69	0.00	0.20	15	
VPPII Vendor 1	-0.0029	0.09	0.27	0.01	0.16	19	
VPPII Vendor 2	-0.0016	0.22	0.19	0.02	0.09	19	
VPPII Vendor 3	-0.0003	0.59	0.05	0.06	0.02	19	

Quality of probe data correlated best with signal density on old VPPI case studies. Visually, the blue line in in Figure 10 shows the positive relation between signal density and percent failed to capture slowdowns, which corresponds a coefficient of determination (i.e., the percent of variation attributed to the least square regression on the independent variable), R^2 , of around 0.41 and a statistically significant slope (low p-value). However, on VPPII case studies (i.e., the yellow/green/blue lines), the relationship is less clear and differs among vendors. The regression plots for VPPII Vendors 1 and 2 (yellow and green) do not show meaningful correlation between probe quality and signal density, a conclusion drawn from low R^2 values (0.03 and 0.02, respectively) and a slope coefficient with an unintuitive sign (i.e., negative slope) that is not statistically significant (indicated by high p-values in Table 6). On the other hand, VPPII Vendor 3 does correlate to slowdown performance and has a statistically significant slope – although its R^2 value is lower than VPPI. Furthermore, a key difference is that VPPII Vendor 3's regression line (red) is shifted much lower than the VPPI line (blue), indicating much better performance

across the range of observed signal densities. Thus, even if there is some drop in performance related to increased signal density (as is seen with VPPII Vendor 3 in the new case studies, and may be expected intuitively), the slowdown performance – measured by percent failed to capture slowdowns – appears to be at least as good as what was observed for low signal densities on old VPPI case studies.

The VPPI case studies showed a negative correlation between AADT and percent failed to capture—although to a lesser extent than signal density, evidenced by an R^2 of 0.20 and marginally statistically significant regression line slope coefficient. While the new case studies for all three vendors also show negative correlations, the goodness of fit is lower for each (R^2 values, of 0.16, 0.09, 0.042, respectively) and the regression line slopes do not appear to be statistically significant for two of three (VPPII Vendor 1's coefficient is marginally significant – similar to VPPI, but even so, the slope is less steep). These results suggest that AADT is not a strong contributing factor to slowdown performance for VPPII probe data. Note that all else being equal, AADT should intuitively be positively associated with performance (i.e., higher volumes mean more opportunities for probe vehicles to characterize traffic conditions) – yet it alone is not reliably associated with probe performance across the range of values observed.

Results by Road Characteristic Range

Finally, the third perspective considers an alternative grouping of slowdowns; rather than considering the results separately for each corridor, it organizes all slowdowns from Tables A.5 and A.6 into ranges of relevant road characteristics (i.e., signal density, AADT) and recomputes slowdown performance for each grouping. The advantage of this approach is that corridors with similar characteristics can be grouped together logically, and within a grouping the computed performance will take all observed slowdowns into consideration (rather than equally weighting performance on each corridor regardless of the number of slowdowns). However, the challenge is determining how to choose ranges for AADT and signal density that are useful. One strategy would be to divide them into ranges with equal number of observations; however, for comparison sake they are separated into ranges that generally correspond to the recommendations made in the initial report.

Previously, the initial arterial report distinguished between signal densities in the following ranges: <= 1 signal per mile, 1-2 signals per mile, and > 2 signals per mile. In this case, we wish to additionally distinguish between 2-3 signals per mile – resulting in four signal density categories for analysis (i.e., 0-1, 1-2, 2-3, >3). For AADT, volumes are separated into the same bins used previously: <=20k vehicles per day, 20k-40k, and > 40k. Tables 8 and 9 summarize the percentage of total slowdowns that correspond to each combination of signal density and AADT bin for VPPI and VPPII case studies, respectively.

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Note that in both tables the "Total" column shows the overall percent of slowdowns observed in each AADT range (0% in 0-20k, 32% in 20-40k, 68% in 40k+ for VPPI; 2% in 0-20k, 31% in 20-40k, 68% in 40k+ for VPPII), while the "Total" Row shows the overall percent of slowdowns observed in each signal density range (24% in 0-1, 40% in 1-2, 8% in 2-3, 28% in 3+ for VPPI; 32% in 0-1, 18% in 1-2, 49% in 2-3, 2% in 3+ for VPPII). Based on how the slowdowns are distributed across road characteristics, there are not enough observations on new case studies to make reliable claims about data quality above 3 signals per mile or below 20k AADT. Thus, subsequent analysis uses the following groupings: 0-1, 1-2, and 2-3 signals per mile for signal density, and 20k-40k and 40k+ for AADT.

Table 8 – Distribution of VPPI slowdowns across road characteristic ranges.

Porce	Percent of Total		Signal Density (Signals / mile)						
Slowdowns		0 - 1	1-2	2-3	3+	Total			
	0 – 20k	-	-	-	-	0			
AADT	20k – 40k	9	-	8	15	32			
AADT	40k +	15	40	-	13	68			
	Total	24	40	8	28				

Values are rounded to the nearest percent

Table 9 - Distribution of VPPII slowdowns across road characteristic ranges.

Dongo	Percent of Total		Signal Density (Signals / mile)						
Slowdowns		0 - 1	1-2	2-3	3+	Total			
	0 – 20k	1	-	1	-	2			
AADT	20k – 40k	3	8	18	2	31			
AADI	40k +	28	10	30	-	68			
	Total	32	18	49	2				

Values are rounded to the nearest percent

Figure 12 summarizes slowdown performance by both signal density and AADT ranges. The left plot focuses on signal density and shows an intuitive trend for old case studies; the slowdown analysis performance clearly decreases as the signal density range increases. However, the trend is less obvious for the new VPPII case studies. While all VPPII vendors show strong performance in the 0-1 signal per mile bin, VPPII Vendors 1 and 2 show moderate overall performance in the 1-2 signals per mile range (roughly 15-25% failed to capture) but perform better in the 2-3 signals per mile bin (generally within 10% failed to capture) – indicating why the regression lines from Figure 10 did not correlate well with signal density. This phenomenon is not evident for VPPII Vendor 3, since its performance is relatively constant in each bin. Given that the 1-2 signals per mile bin only contains about 18% of all slowdowns (see Table 9), challenges experienced on a single corridor may be artificially lowering the performance for Vendors 1 and 2. In any case, is important to emphasize that data in the 2-3 signals per mile range – which previously was deemed unreliable based on old case studies – appears to now be much more accurate than before.

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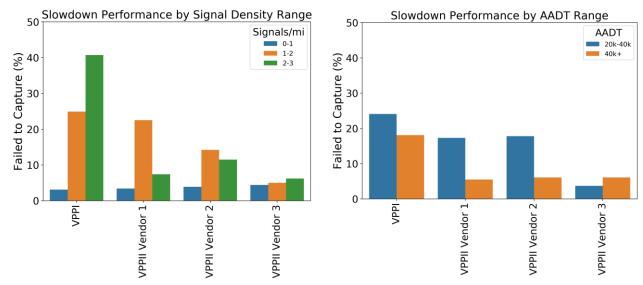


Figure 12 – Slowdown performance by signal density (left) and AADT (right) ranges.

The right plot in Figure 12 focuses on slowdown performance by AADT bin. In old case studies, the performance improves a bit as volumes increase between 20k-40k and 40k+bins. This trend is even more noticeable for VPPII vendors I and II, but does not hold for VPPII Vendor 3, whose performance slightly decreases for higher volumes. However, it is important to note that the 20k-40k and 40k+ volume bins do not have the same number of slowdowns in each bin – nor the same proportion of slowdowns with different signal density characteristics. In other words, it is likely that the moderate performance observed by VPPII Vendors I and II in the 20k-40k range is related to other factors (as opposed to being solely a result of volume level). In fact, further investigation showed that the failed to capture slowdowns that contributed to this trend occurred at locations where there are 1-2 signals per mile (i.e., the same locations that caused the drop in performance for the 1-2 signals per mile bin in the left plot). Thus, while the results indicate that the performance on high volume roads is strong, it is unlikely that volume level alone is the determining factor – at least in the observed volume ranges.

5. Conclusions

This report provides an update to the original arterial report, quantifying the accuracy of VPPII data across 13 additional arterial datasets and two additional vendors, and comparing these results to the original VPPI case studies. These results indicate that **probe data performance on arterials has improved since the original analysis in 2014**. In particular:

- Performance on the traditional analysis has improved, and VPPII probe data for all three vendors is much more consistently within contract specifications for traditional error measures (i.e., AASE and SEB) than was observed in the original VPPI case studies in 2013-2014. This is improvement is observable both when comparing vendor speeds to the ground truth Mean and Standard Error of the Mean (SEM) band speeds.
- Performance on the slowdown analysis the current preferred way to quantify operational performance on arterials improved dramatically relative to the VPPI levels for all three vendors and is less strongly linked to AADT and signal density. The average percentage of slowdowns missed by each vendor (an indicator of slowdown performance) across corridors dropped by over 50%, resulting in improved performance across a range of road characteristics.

Note that the case studies represent a finite range of road characteristics (e.g., AADT, signal density), and the conclusions may not generalize to conditions outside the observed data (e.g., roads with extremely low volumes or high signal densities far beyond what was observed in the case studies). However, within the observed range of observed road conditions – particularly 0-3 traffic signals per mile and above 20k AADT, all three vendors typically perform at a level that is suitable for planning and many operational applications. This is a noticeable improvement from the results shared in the previous report, where performance was more closely linked to road characteristics and degraded significantly for signal densities over 2 signals per mile.

Nonetheless, **arterial roads still present certain fundamental challenges**, and should be treated with caution. Some fundamental issues include:

• Probe data consistently errors toward faster speeds during congested periods, although to a lesser extent than was previously observed in the initial arterial report. This phenomenon can be observed in the lowest speed bins for all vendors in Figure 8, which show positive Speed Error Bias assessed against both the Mean and SEM band. While there is a clear positive bias in the lowest speed bin (i.e., during congestion) for all vendors, the magnitude of the average bias is less for all

three VPPII vendors relative to the VPPI levels. As the initial report noted, one consequence of overreporting speeds during congestion is that it may appear that congestion is getting worse in the future as probe vendors improve their reporting capabilities and better capture the extent of slowdowns

• Complex flow patterns common on signalized roadways cannot be observed in VPP data. Arterials with many traffic signals can produce multi-modal speed distributions due to signal timing patterns. While the validation metrics may still be within contract specifications because the ground truth data has a large confidence band of "acceptable" ground truth mean speeds, the issue remains that VPP data only reports a single speed, which may be sufficiently accurate for some applications, but not others. Note that the original report also noted that VPP data tended to track the higher speed (i.e., "optimistic bias"), but this was not noticed to the same extent in the new case studies. In any case, when actual traffic conditions are complex and there are distinct modes, the VPP data can only track one of the modes or report a value in between them, which may not reflect the speed of any vehicles.

Accordingly, even as probe data is becoming increasingly useful on higher signal density roads (as evidenced by strong slowdown analysis performance in the 2-3 signals per mile range), it is important to remember that traffic signals still cause complex traffic patterns that cannot be fully observed by VPP data – especially during typical (i.e., non-slowdown) conditions. Thus, roads with high traffic signal density should still be used with caution.

• Low volume roads are difficult to validate: Extremely low volume roads pose challenges for VPP vendors and validation efforts because both VPP probe data and ground truth re-identification data rely on sampling techniques. This means data vendors have fewer probe vehicle observations to generate VPP speed data, and in some cases the validation team cannot collect enough data to meaningfully evaluate the probe data over 5-minute time periods (e.g., there may not be enough observations to quantify the ground truth conditions, or the confidence band may be extremely large). Furthermore, few VPPII case studies have been conducted on roads with less than 20k AADT, so the validation team has not been able to thoroughly evaluate data quality in this range. Thus, even though AADT does not correlate strongly to data accuracy in the ranges observed in this report, low volume situations should be treated with caution – particularly when used for operational purposes.

Note that the evaluation techniques employed in this study evaluate data quality over short periods of time, thus emphasizing operations and performance

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management applications. Accordingly, concerns about being able to effectively quantify ground truth conditions on low volume roads are also based on this perspective. However, it is worth pointing out that probe data can also be used for planning purposes, which tends to focus on larger temporal periods - in which case the challenges associated with low-volume roads may be less pronounced (e.g., data could be aggregated over a larger temporal window to generate sufficient observations for analysis). VPPII probe data has not been explicitly studied in this manner due to lack of low-volume case studies but may be considered in future research.

Despite these fundamental challenges, VPPII arterial speed data is more accurate than what was observed in the previous report, able to better capture traffic slowdown events, and suitable for use over a wider range of road characteristics. Based on these findings, the validation team makes the following recommendations to the I-95 Corridor Coalition and its members:

- The I-95 Corridor Coalition should continue to evaluate probe data quality on arterials to benchmark vendor capabilities. It is expected that probe data will continue to improve over time, but the I-95 CC should continue to emphasize arterial validations to regularly track vendor performance.
- Additional emphasis should be placed on refining analysis techniques for evaluating data quality on arterials. While capable of providing some useful insight, the traditional validation analysis approach should not be the only technique used to quantify data accuracy on arterials. Currently, the slowdown analysis is seen as the best way to quantify probe data accuracy for operational purposes, and thus it should continue to be developed. It may be useful to explore whether it is possible to partially automate the slowdown analysis process to decrease reliance on an analyst's judgement for the classification process. Additionally, future work should consider whether existing arterial performance management analysis tools and performance measures can be leveraged for the purposes of probe data validation on arterials. Finally, it would be useful to evaluate probe data on low volume roads, which may mean introducing additional techniques to quantify typical performance over longer temporal periods.
- The I-95 Corridor Coalition should engage probe data vendors to discuss whether additional information can be reported on arterials. The validation team

 in cooperation with Coalition members should consider what information should be reported in addition to average travel time / speed. As previously discussed, signalized arterials have complex traffic patterns that cannot be fully characterized by a single speed value.

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Appendix

Tables A.1-A.4 report the traditional error metrics for both old and new case studies, while Tables A.5-A.6 summarize the slowdown analysis results.

Table A. 1 – Traditional AASE and SEB error metrics for old case studies (VPPI)

		Co	mparison v	vith Mean /	Comparison with SEM Band				
Data Set	Av	erage Absolı (AA	-	rror	Speed Error Bias (SEB)				
(State-		Spee	d Bin			Spec	ed Bin		
ID#)	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH	
NC-06	13.9/9.6	10.9/3.4	5.6/1	5.1/1.3	13.7/9.6	10.5/3.3	3.8/0.7	-3.4/-1.1	
MD 07	12.7/6.4	7.3/3	3.7/0.9	12.5/6.3	12.7/6.4	7/2.9	1.3/0.3	-12.4/-6.2	
MD-07	13.5/7.5	8.6/3.4	4.7/1.2	7.8/2.9	13.4/7.5	8.2/3.3	2.3/0.8	-7.3/-2.8	
	4.4/2.9	7.3/5.3	9.6/5.4	6.5/2.3	3.8/2.8	6.9/5.2	8.8/5.2	-2.9/-1.3	
NJ-11	13.4/7.4	10.3/3.6	6.4/1.3	5.6/1.8	13.4/7.4	10.1/3.6	5.3/1.1	-4/-1.6	
	19.9/12.2	13.8/5.1	7.1/2.8	4.9/1.5	19.9/12.2	13.7/5.1	6.5/2.6	-1.4/-0.6	
NII 10	12.8/9.5	11.8/7.5	7.7/3.1	4.9/1.5	12.8/9.5	11.7/7.5	7.2/3	-1.2/-0.6	
NJ-12	7/4.7	9/4.1	7.5/3.5	5.2/1.8	7/4.7	8.8/4	6.4/3.2	0/-0.1	
DA OF	11.5/7.5	8.4/4.7	5.7/1.9	4.9/1.3	11.3/7.5	8.2/4.7	4.6/1.7	-2.7/-0.9	
PA-05	8.2/5.9	8.3/4.3	6.7/2.9	3.1/1.3	8.1/5.9	8.2/4.3	6.3/2.7	-0.1/0	
DA 06	9.1/4.9	6.1/2.9	4.1/1.1	6.1/2.3	9.1/4.9	5.7/2.8	1.7/0.6	-4.4/-1.7	
PA-06	6.5/3.3	3.4/1.3	5.3/2.1	12.9/5.5	6.4/3.3	1.5/0.8	-4.9/-2	-12.9/-5.5	
	10.8/7.5	7.8/4.5	6.7/2.6	6.3/2.2	10.7/7.5	7.3/4.3	4.8/2.1	-3.5/-1.6	
VA-07	19.3/18.3	13.8/10.8	12.4/6.6	5.2/1.7	19.1/18.2	13.4/10.6	12/6.5	-0.9/-0.6	
	8.1/3.9	3.7/1.2	2.5/0.6	8.7/5.2	8/3.8	2.8/1	-1.1/-0.5	-8.7/-5.2	
VA-08	11.9/7.2	7.4/3.8	6.8/1.3	6.2/2.5	10.7/7.1	7.4/3.7	1.8/0.5	-3.9/-1.2	
MD 00	9.7/5.8	5.1/1.9	4.1/1.1	9.2/3.8	9.6/5.8	4.2/1.8	-1.5/-0.4	-9/-3.8	
MD-08	18.9/16.9	13.1/10.1	13.5/6.8	6.6/2.9	18.9/16.9	12.9/10.1	12.5/6.2	-2.9/-1.6	

Table A. 2-Traditional AASE and SEB error metrics for new case studies (VPPII Vendor 1)

		Co	mparison w	ith Mean / (Comparison	with SEM B	and		
Data Set	Av	erage Absolı (AA	_	ror	Speed Error Bias (SEB)				
(State-	Speed Bin				Speed Bin				
ID#)	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH	
VA-09	7.8/4.8	4.6/1.4	3.9/1.1	7.8/3.8	7.8/4.8	3.6/1.2	-1.5/-0.6	-7.4/-3.8	
VA-10	7.3/4.6	4.7/1.6	4.4/1.1	6.0/2.3	7.1/4.6	3.2/1.2	-0.2/0.0	-4.5/-2.0	
NJ-13	7.0/5.1	7.2/2.8	5.1/0.9	5.4/1.3	7.0/5.1	6.9/2.8	3.3/0.7	-3.3/-1.2	
NC-07	6.8/4.7	4.6/2.1	3.2/1.1	4.2/1.9	6.8/4.7	4.3/2.1	2.1/0.7	-3.5/-1.8	
GA-02	5.3/3.7	5.3/2.3	3.6/0.7	4.8/1.7	5.1/3.6	4.7/2.1	1.2/0.3	-3.9/-1.5	
MD-10	7.2/4.2	5.6/1.3	3.8/0.9	6.8/2.3	7.2/4.2	5.0/1.2	0.7/0.4	-4.3/-1.9	
PA-09	4.1/2.1	2.8/0.8	3.4/0.9	7.9/2.4	4.1/2.1	1.2/0.4	-2.0/-0.7	-7.7/-2.4	
VA-11	5.0/2.9	6.3/2.4	5.2/1.2	5.8/1.8	4.6/2.8	5.2/2.3	2.0/0.5	-3.8/-1.6	
NJ-14	5.0/2.8	4.8/1.8	5.2/1.5	5.9/1.7	4.9/2.8	3.1/1.6	2.2/0.8	-3.5/-1.3	
MD-12	9.6/3.1	5.5/1.3	5.2/1.3	6.6/2.7	9.6/3.1	4.8/1.2	1.1/0.3	-5.2/-2.6	
NC-09	6.1/2.7	5.6/1.6	6.0/1.3	5.8/2.2	5.9/2.7	4.7/1.4	3.2/0.6	-3.4/-1.9	
GA-04	3.9/1.64	5.15/1.49	5.33/1.15	4.88/1.58	3.06/1.44	3.48/1.03	2.66/0.33	-2.74/-1.38	
PA-11	9.59/3.13	5.53/1.28	5.2/1.34	6.57/2.7	9.58/3.13	4.82/1.2	1.07/0.34	-5.2/-2.56	

Table A. 3 – Traditional AASE and SEB error metrics for new case studies (VPPII Vendor 2)

	Comparison with Mean / Comparison with SEM Band									
Data Set (State- ID#)	Ave	erage Absolu (AA	-	ror	Speed Error Bias (SEB)					
	Speed Bin				Speed Bin					
	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH		
VA-09	9.5/ 6.4	6.6/2.8	5.1/1.7	6.9/3.2	9.5/6.4	5.7/2.6	0.6/0.2	-5.1/-2.6		
VA-10	11.8/8.8	7.2/3.3	5.4/1.7	5.6/2.1	11.7/8.7	5.8/2.9	2.4/0.8	-3.0/-1.4		
NJ-13	6.7/4.8	9.7/5.0	8.1/2.8	6.0/1.6	6.4/4.8	8.9/4.8	6.3/2.4	-0.4/-0.5		
NC-07	4.2/2.7	4.0/1.8	4.7/2.0	4.7/2.3	3.8/2.6	1.8/1.1	2.0/0.9	-3.1/-1.8		
GA-02	4.8/3.2	7.5/4.2	6.1/2.2	4.4/1.5	4.5/3.1	7.0/4.0	4.3/1.8	-1.7/-0.8		
MD-10	8.6/5.6	9.0/3.6	6.1/2.3	7.0/2.4	8.5/5.5	8.4/3.5	4.0/1.8	0.2/-0.2		
PA-09	5.4/3.4	4.0/1.7	4.2/1.3	6.9/2.0	5.2/3.3	2.2/1.1	-0.2/-0.1	-5.4/-1.8		
VA-11	6.9/4.6	7.5/3.4	6.6/2.0	6.7/2.4	6.7/4.6	6.2/3.2	2.5/0.9	-3.4/-1.8		
NJ-14	5.4/3.6	5.9/3.0	6.3/2.5	5.9/1.7	5.2/3.6	4.9/2.8	4.1/2.0	-1.7/-0.7		
MD-12	13.2/6.1	8.7/2.8	6.1/1.8	6.5/2.4	13.2/6.1	8.1/2.6	3.7/1.3	-3.6/-1.9		
NC-09	8.1/4.4	7.1/2.5	7.4/2.0	5.9/2.0	8.1/4.4	6.3/2.3	5.1/1.4	-1.6/-1.4		
GA-04	4.47/1.94	5.52/1.7	6.47/1.79	5.03/1.55	3.78/1.74	4.08/1.3	4.08/1	-1.7/-1.13		
PA-11	13.25/6.08	8.74/2.8	6.07/1.85	6.46/2.42	13.19/6.07	8.06/2.61	3.67/1.26	-3.64/-1.95		

Table A. 4 - Traditional AASE and SEB error metrics for new case studies (VPPII Vendor 3)

	Comparison with Mean / Comparison with SEM Band										
Data Set	Av	erage Absolı (AA	-	rror	Speed Error Bias (SEB)						
(State-	Speed Bin	l			Speed Bin						
ID#)	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH	0-15 MPH	15-25 MPH	25-35 MPH	>35 MPH			
VA-09	12.3/9.0	11.3/6.1	7.6/3.3	5.2/1.8	12.3/9.0	11.2/6.1	6.8/3.1	0.1/-0.2			
VA-10	9.6/6.7	9.9/5.4	7.9/3.3	6.2/2.5	9.4/6.6	9.6/5.3	6.7/3.0	-1.2/-1.1			
NJ-13	6.6/4.7	12.1/7.0	9.0/3.7	5.9/1.3	6.6/4.7	12.1/7.0	8.5/3.6	0.3/-0.4			
NC-07	7.5/5.4	7.8/4.5	7.5/3.8	3.8/1.5	7.4/5.4	7.6/4.5	6.9/3.6	-1.2/-0.8			
GA-02	5.9/4.3	10.1/6.3	10.3/5.2	5.5/2.0	5.8/4.3	9.7/6.2	8.7/4.6	-3.5/-1.5			
MD-10	9.4/6.3	12.6/6.3	8.3/3.7	6.5/2.0	9.3/6.3	12.5/6.3	7.6/3.6	-0.7/-0.6			
PA-09	6.1/3.8	6.0/3.3	4.5/1.5	5.1/1.1	6.0/3.8	5.7/3.2	2.4/1.0	-0.1/-0.2			
VA-11	6.4/4.1	8.5/4.2	6.6/1.9	5.9/1.9	6.3/4.1	7.9/4.1	4.0/1.4	-3.0/-1.5			
NJ-14	3.9/1.8	4.1/1.3	4.4/1.1	6.3/2.0	3.7/1.8	2.0/1.0	1.3/0.6	-4.1/-1.6			
MD-12	8.7/2.4	5.5/1.2	5.2/1.6	6.3/2.4	8.7/2.4	5.0/1.2	2.5/1.0	-4.4/-2.3			
NC-09	6.6/3.0	6.7/2.2	7.7/2.1	5.2/1.4	6.5/3.0	6.1/2.1	6.0/1.7	-0.6/-0.9			
GA-04	3.42/1.27	5.42/1.46	4.97/1.08	4.21/1.02	3.1/1.21	3.84/0.96	2.89/0.45	-1.26/-0.73			
PA-11	8.75/2.37	5.55/1.21	5.23/1.57	6.27/2.44	8.75/2.37	5.04/1.18	2.55/1.05	-4.38/-2.31			

Table A. 5 - Slowdown analysis results on old case studies (VPPI)

Corridor	Average AADT	Average Signal	Slowdown Analysis					
Code	(in 1000)	Density	Total Slowdowns	% Fully Captured	% Partially Captured	% Failed to Capture		
1	25.0	2.1	54	14.8	44.4	41.3		
2a	44.0	3.9	9	44.4	22.2	33.3		
2b	34.0	3.1	8	0.0	37.5	62.5		
3a	70.0	0.7	101	63.4	36.6	0.0		
3b	48.0	1.8	4	0.0	100.0	0.0		
3c	42.0	2.0	4	25.0	50.0	25.0		
4a	46.0	1.8	57	40.4	38.6	21.1		
4b	52.0	1.7	89	41.6	46.1	12.4		
5a	45.0	4.1	78	28.2	48.7	23.1		
5c	25.0	0.5	58	50.0	41.4	8.6		
6a	27.0	3.3	18	22.2	33.3	44.4		
6b	21.0	11.5	5	0.0	20.0	80.0		
7a	56.0	1.9	75	24.0	42.7	33.3		
7b	55.0	1.6	22	4.5	22.7	72.7		
7c	21.0	5.0	1	0.0	0.0	100.0		
8	33.0	3.6	49	8.2	42.9	49.0		
9a	31.0	3.9	20	0.0	35.0	65.0		
9b	42.0	1.2	18	22.2	66.7	11.1		

Table A. 6 - Slowdown analysis results on new case studies (VPPII)

Corridor Code	Average AADT (in 1000)	Average Signal Density	Vendor#	Slowdown Analysis					
				Total Slowdowns	% Fully Captured	% Partially Captured	% Failed to Capture	% Missing Data	
			1		59.5	29.1	11.4	0.0	
11	36.0	2.9	2	79	40.5	43.0	16.5	0.0	
			3		43.0	41.8	2.5	12.7	
			1	18	27.8	22.2	50	0	
12	22.0	1.2	2		27.8	0.0	72.2	0.0	
			3		33.3	55.6	0.0	11.1	
			1		29.4	58.8	0	11.8	
13	39.8	1.0	2	17	52.9	41.2	5.9	0.0	
			3	-	41.2	58.8	0.0	0.0	
			1		4.5	36.4	59.1	0.0	
14a	28.7	1.4	2	22	40.9	54.5	4.5	0.0	
			3		50.0	31.8	18.2	0.0	
			1		65.8	26.6	7.6	0.0	
14b	57.8	1.0	2	79	96.2	3.8	0.0	0.0	
	07.0		3		48.1	41.8	10.1	0.0	
15a	43.2	2.3	1	56	71.4	25.0	0.0	3.6	
			2		83.9	12.5	0.0	3.6	
			3		67.9	28.6	3.6	0.0	
	30.8	1.9	1	11	54.5	45.5	0.0	0.0	
15b			2		81.8	18.2	0.0	0.0	
			3		63.6	36.4	0.0	0.0	
15c	146.5	0.0	1	68	98.5	1.5	0.0	0.0	
			2		88.2	11.8	0.0	0.0	
			3		91.2	7.4	1.5	0.0	
	29.2	2.2	1	8	62.5	12.5	25.0	0.0	
16a			2		75.0	12.5	12.5	0.0	
			3		100.0	0.0	0.0	0.0	
			1		86.7	13.3	0.0	0.0	
16b	62.0	1.5	2	30	86.7	13.3	0.0	0.0	
	02.0	1.0	3		90.0	10.0	0.0	0.0	
		4.6	1		90.9	9.1	0.0	0.0	
17a	28.3		2	. 11	27.3	63.6	9.1	0.0	
			3		45.5	36.4	18.2	0.0	
			1		0.0	25.0	75.0	0.0	
17b	11.1	0.9	2	. 8	12.5	62.5	25.0	0.0	
	11.1		3		87.5	12.5	0.0	0.0	
		4.6	1		0.0	33.3	66.7	0.0	
17c	23.4		2	. 3	66.7	0.0	33.3	0.0	
			3		66.7	0.0	33.3	0.0	
	1		1		72.1	19.3	8.6	0.0	
18	52.9	2.8	2	140	32.9	55.7	11.4	0.0	
10	32.9	2.0	3	140	59.3	30.0	10.7	0.0	
19a	74.1	1.9	1	39	51.3	35.9	12.8	0.0	

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Corridor	Average AADT (in 1000)	Average Signal Density	Vendor#	Slowdown Analysis					
Code				Total Slowdowns	% Fully Captured	% Partially Captured	% Failed to Capture	% Missing Data	
			2		51.3	41.0	7.7	0.0	
			3		41.0	20.5	5.1	33.3	
			1		58.1	38.7	3.2	0.0	
19b	90.4	0.9	2	31	35.5	41.9	22.6	0.0	
			3		61.3	38.7	0.0	0.0	
			1		70.0	30.0	0.0	0.0	
19c	75.5	0.4	2	10	70.0	30.0	0.0	0.0	
			3		100.0	0.0	0.0	0.0	
	40.3	2.4	1	4	50.0	25.0	25.0	0.0	
20			2		25.0	25.0	50.0	0.0	
			3		25.0	75.0	0.0	0.0	
	28.9	2.1	1	20	70.0	30.0	0.0	0.0	
21			2		60.0	20.0	20.0	0.0	
			3		65.0	35.0	0.0	0.0	
	31.9	2.7	1	11	81.8	18.2	0.0	0.0	
22a			2		72.7	18.2	9.1	0.0	
			3		90.9	0.0	9.1	0.0	
	22.2	2.3	1	5	80.0	20.0	0.0	0.0	
22b			2		80.0	20.0	0.0	0.0	
			3		100.0	0.0	0.0	0.0	
		5.7	1		-	-	-	-	
22c	20.9		2	0	-	-	-	-	
			3		-	-	-	-	
		2.4	1		75.0	25.0	0.0	0.0	
23	15.7		2	4	75.0	25.0	0.0	0.0	
			3		100.0	0.0	0.0	0.0	